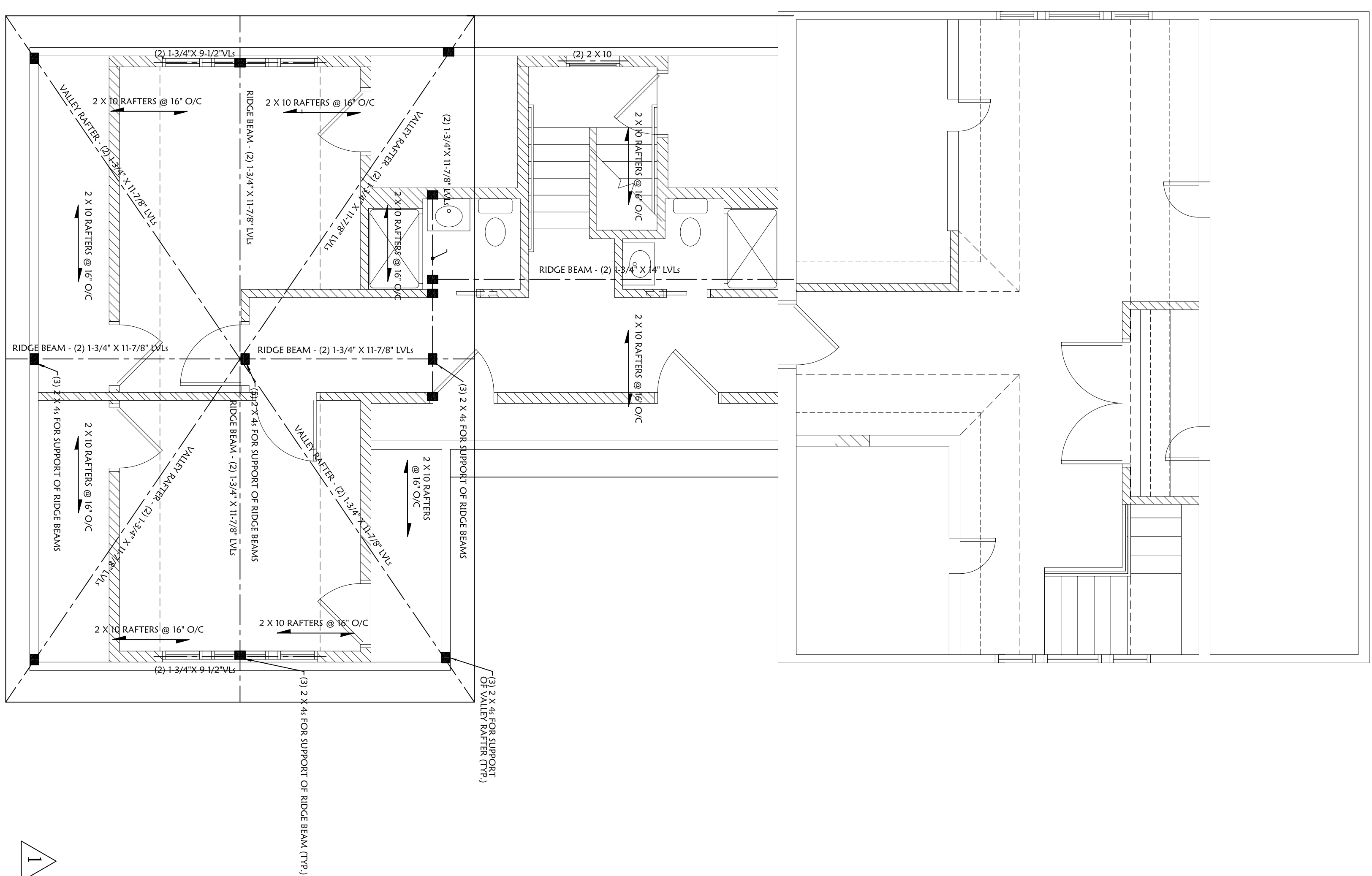


SECOND FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

1 REGION REFLECTS CHANGES TO FLOOR FRAMING AND ROOF FRAMING UNLESS SPECIFICALLY INDICATED

GENERAL NOTES:

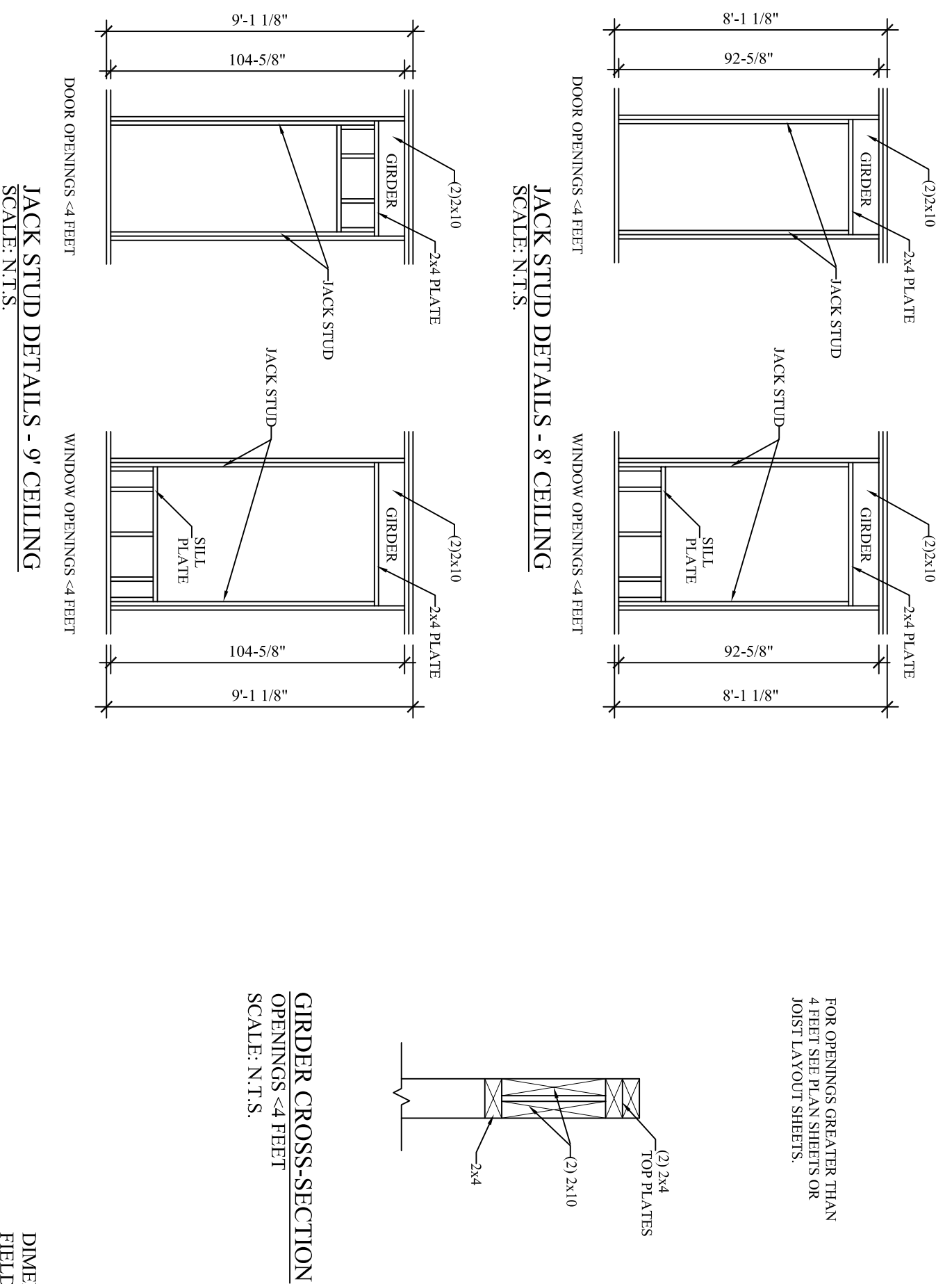
1. GOVERNING CODES ARE THE 2006 SERIES INTERNATIONAL CODES.
2. THE DESIGN LOADS SHALL BE AS FOLLOWS:
 - A) ROOF LIVE LOAD
 - 1) NO ROOF LIVE LOAD OTHER THAN SNOW
 - B) ROOF SNOW/LOAD
 - 1) GROUND SNOW LOAD - 40 PSF
 - 2) FLAT ROOF SNOW LOAD - 40 PSF
 - 3) SNOW EXPOSURE FACTOR (C_e) - 0.9
 - 4) SNOW LOAD IMPORTANCE FACTOR (I) - 1.0
 - 5) THERMAL FACTOR (C_t) - 1.0
 - C) WIND DESIGN DATA
 - 1) BASIC WIND SPEED (3 SECOND GUST) - 90 MPH
 - 2) WIND IMPORTANCE FACTOR (I) = 1.0
 - 3) WIND EXPOSURE CATEGORY B
 - 4) COMPONENTS AND CLADDING:
 - P NET. WALLS 5 PSF, +13.0 PSF
 - P NET. ROOFS 1 PSF, +6.7 PSF
 - D) EARTHQUAKE DESIGN DATA
 - 1) SEISMIC IMPORTANCE FACTOR = 1.0
 - 2) MAPPED SPECTRAL RESPONSE FACTORS:
 - $S_S = 0.16g$
 - $S_1 = 0.06g$
 - 3) SITE CLASS B
 - 4) SPECTRAL RESPONSE COEFFICIENTS:
 - $S_{D5} = 0.11g$
 - $S_{D1} = 0.04g$
 - 5) SEISMIC DESIGN CATEGORY = A
 - 6) BASIC SEISMIC-RESISTING-FORCE SYSTEMS: DIAPHRAGM ACTION OF WALLS AND ROOF
 - 7) DESIGN BASE SHEAR - 40 KIPS
 - 8) RESPONSE MODIFICATION FACTOR:
 - R = 6.5
 - 9) ANALYSIS PROCEDURE USED:
 - IBC 1616.4

1E) CONVENTIONAL LIGHT FRAME CONSTRUCTION IN ACCORDANCE WITH SECTIONS 2304 & 2308.

- 1E) DESIGN LOADS:
 - PRIVATE ROOMS & CORRIDORS: 40 PSF
 - DEAD LOAD: 15 PSF

3. DESIGN SOIL BEARING CAPACITY = 2500 PSF

INTERIOR WALL LOAD BEARING HEADERS/GIRDER DETAILS



General Notes:

1. All construction shall be done in conformance with the following standards:
 - Concrete work - American Concrete Institute
 - Masonry - National Concrete Masonry Association
2. Design Data:
 - Soil bearing capacity - 2500 PSF
 - Concrete piers - $f_c = 4000$ psi
 - Concrete footings - $f_c = 3000$ psi
 - Concrete slabs on grade - $f_c = 3500$ psi
 - Reinforcing steel - $F_y = 60$ ksi
3. All concrete shall be stone or gravel aggregate to test as specified at 28 days. All concrete exposed to weather shall be air entrained.
4. All detailing, fabrication and erection of reinforcing bars shall be in strict accordance with ACI "Manual Standard Practice for Detailing Reinforced Concrete Structures (ACI 315, latest edition).
5. Splices in continuous reinforcing, column verticals, wall bars, dowels, etc., shall be lapped 30 diameters. In groups of parallel bars, splices shall be staggered.
6. Slabs on grade shall have 6 x 6 - W1.4 / 1.4 welded wire reinforcing mesh unless otherwise indicated on the drawings. All mesh shall be lapped a minimum of 6 inches. See drawings for construction and control joint locations.
7. Where reinforcing is specified for a certain part of the building, it shall be duplicated in similar parts of the building, unless otherwise noted.
8. Unless noted otherwise, size and reinforcing for continuous footings under masonry walls shall be 12" deep, 6" projection minimum from each face of wall, with one #4 bar for each 8" of footing width. Provide transverse reinforcing in the form of #4 bars at 32" on center when footing width exceeds 32".
9. Concrete reinforcing protections shall be as follows:
 - a) Bottom of footings, concrete cast against earth - 3"
 - b) Piers, walls exposed to ground or earth - 1-1/2" for #5 and smaller, 2" for #6 and larger.
10. All conduit and piping shall be individually sleeved through concrete work.
11. Provide expansion joint material where slabs abut walls, columns, and other vertical surfaces, unless noted otherwise.
12. All mortar shall be Type S and shall conform to ASTM C270.

EXTERIOR WALL HEADERS/GIRDER DETAILS

